

Track Design Optimization for Very High Speed FFE (Madrid, Spain) – 21 September 2017

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• Outline

- 1. Objectives and framework
- 2. Computational model
- 3. Experimental tests
- 4. Model validation
- 5. Track response overview for VHS
- 6. Track design optimization for VHS
- 7. Final remarks







Objectives and framework

Task 1.2.2 - "Track design for VHST" aims at proposing innovative **optimized track design for very high speed** on the basis of:

- numerical simulation (IST)
- real scale laboratory test CEDEX (Track box CTB)
- Computational model validated based on the data provided by CEDEX (Track box ballasted track)
- Computational model used to predict dynamic response of the reference railway track (CTB) and evaluate influence of train speed increase up until 400 km/h.
- ➤ Numerical simulations made to evaluate influence of track design variants (mainly specific combinations of railpads and USPs suggestions in collaboration with the other partners as CEDEX, Adif) → support CEDEX to prepare the experimental tests to be done.
- Computational model used to perform guided numerical simulations towards VHST design optimization aiming at reduced vibrations inside track and slower track degradation.





Which makes possible to effortlessly attain:

 Simulations of millions of HS trains passages to predict track settlement progression along time





Computational model

Computational developments for **long-term predictions** of track geometric deterioration

- 1. Introduction of initial defects in the track
- 2. Track settlement increases along time, after millions of running cycles
- 3. Model enables to know track longitudinal configuration at each cycle and its





• Experimental Tests

CEDEX Track Box (CTB) – Reference Track case

Trackbox section with granular subballast used for validation with results from model

Railway components

Component type		Description	
Rails		UIC 60-E1	
Rail pads			
	Туре	PAE2	
	Thickness	7 mm	
	Area	148×180 mm2	
	Nominal stiffness	Around 100 kN/mm	
	Secant stiffness	Between 20 kN/mm and 9	95 kN/mm
Sleepers			
	Туре	AI-99 (Monoblock)	
	Mass	344 kg (average)	source: (CEDEX, 2015)

Material properties of the supporting layers

	h	Е	ν	Density
	[m]	[MPa]	[-]	[kN/m³]
Ballast	0,40	230	0,20	17,0
Granular sub ballast	0,30	440	0,30	22,0
Form layer	0,60	400	0,30	21,5
Embankment	2,57	385	0,40	20,2









Track box: cross-section schematic representation with sensors location





Model validation

Comparison of Experimental and Computational Results

Static and Quasi-Static Tests





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rail vertical displacement time signals



ballast vertical displacement time signals







Model validation

Comparison between experimental measurements CTB and dynamic calculations at 300km/h













• Track response overview for VHS

Track response overview: key aspects

- Input parameters: rail pad and USP vertical stiffness
- Design space bounds

Rail pads: 20 kN/mm to 200 kN/mm

USPs: 20kN/mm to 500 kN/mm

- Circulation speeds: 300, 350 and 400km/h.
- Multi-objective minimization problem

Minimize peak vertical acceleration levels within ballast layer and sleepers

- DIRECT algorithm (IST version with support for multi-objective optimization)
- Simulation data used generate response surfaces



















Track response overview for VHS Remarks Influence of rail pad and USP vertical stiffness variations (k_{pad}, k_{usp}) Peak sleeper acceleration Decrease with increased k_{usp} Sensitivity is higher when k_{usp} is low Peak values are very high when k_{usp} is low Qualitatively, the link between peak vertical vibration levels and the design parameters (vertical stiffness of rail pads and USPs) is observed on all circulation speeds. Peak ballast acceleration Insensitive to k_{usp} , except when k_{usp} values are very low Sensitive to k_{pad} Caution No real in situ data on USP tracks No measurements from VHS trains at 400km/h





• Track design optimization for VHS

TEST plan support CEDEX to prepare tests in the CTB; coordination with other partners

- Track design variants to be tested: combinations of Railpads+USPs
 - **Compute dynamic** simulations of the passage of a train circulating at high and very-high speeds on each track variant
 - Evaluate peak acceleration levels observed in the track (in particular ballast layer and on the sleepers)
- Optimized Design to enhance track dynamic performance, controlling and reducing track vibrations with speed increase

Test reference	Kpad [kN/mm]	Kusp [kN/mm]	Variant description	Notes	
Test 1	100	-	M(100,-)	Reference Track box CTB	
Test 2	40	-	M(40,-)		
Test 3	60	-	M(60,-)		
Test 4	40	80	M(40,80)	Test 2 + USP	
Test 5	60	80	M(60,80)	Test 3 + USP	
	80	80	M(80,80)	Additional variant	
Test 1 + USP	100	80	M(100,80)	Reference CTB + USP	
	60	200	M(60,200)	Additional variant	
	60	500	M(60,500)	Additional variant	
	80	50	M(80,50)	Additional variant	
	100	50	M(100,50)	Additional variant	
1 -			M(k _{pad} , k _{usp})	



• Track design optimization for VHS Parametric study Rail pad stiffness k_{pad} [kN/mm] 40, 60, 80, 100 Railpad+USP USP stiffness k_{usp} [kN/mm] 40, 60, 80, 100 Train speed v [km/h] 300, 320, 330, 350, 360, 380, 400 Steper (top)













Track design optimization for VHS Parametric study Relative reductions in peak **ballast** accelerations with regards to the reference model Speed: 400 km/h Speed: 350 km/h Speed: 300 km/h -9.2% -16.8% Peak ballast acceleration $[m/s^2]$ -18.7% Peak ballast acceleration [m/s²] Peak ballast acceleration [m/s²] -27.9% -16.9% -18.2% -30.2% -30.2% -14.3% 3 3 3 -22.3% -27.8% -26.6% 2 2 2 Reference M(40,60) M(40,80) M(60,80) M(80,100) Reference M(40,60) M(40,80) M(60,80) M(80,100) Reference M(40,60) M(40,80) M(60,80) M(80,100) **FÉCNICO**





• Final Remarks

- ✓ The introduction of USPs results in a significant reduction in peak vertical displacement and acceleration levels within the track supporting layers, ballast layer included, for all the track design solutions tested.
- ✓ However, it must be highlighted that these improvements are accompanied by increases in peak vertical displacement and acceleration levels on track components supported by the USPs, as the rails and the sleepers.
- ✓ Notwithstanding, the results also suggest that incorporating stiffer USPs may reduce peak acceleration levels within the ballast layer while preserving peak sleeper acceleration levels.







• Final Remarks

Along the interpretation and critical analysis of the results attention must be paid to the following:

- ✓ The numerical model is not able to consider the following positive effects known already to be provided by USPs:
 - increase in the interface and load-distributing area between sleepers and ballast
 - embedding effect of the ballast stones by the USP elastic layer
- ✓ Any results obtained for trains speeds of 400km/h must be taken with care as no validation with real measurements was made at these speeds;
- ✓ The numerical results here analysed are provided exclusively from short term computations, that is, only track instantaneous responses are obtained, so, conclusions cannot be directly extrapolated to track long-term performance nor within a life cycle analysis perspective;







Thank you for your kind attention



